PileCap.exe User Manual

General

The PileCap.exe PC-program is used for the analysis of 3-D pile groups cast into a rigid cap. The soil and the pile top conditions (fixed or hinged) can be considered in the calculations. The number of piles and loading cas es are only restricted by the resources of the user.

Pile/Soil/Load –data are input by means or a user friendly interface and modifications of the task are easy to implement. In this way, the optimum design of a pile group within given conditions can be obtained in an interactive manner, fast and effective

The method for analysis, direct stiffness method, is well proven for pile group design. The output is all the time checked for numerical stability and accuracy, meaning the user can feel confident that the results calculated. In case of attempts to analyse an instable structure, the application will issue a warning.

Soil and pile properties are easy and hands on to input. It is possible to design a pile group with every pile different from their neighbours, although this is of course never the case in practice. Nevertheless, the ability to analyse a pile group with one, or several, piles different from the others is many times of great value. Pile group analysis after installation and checking the piles, when unenviable deviations from original design usually shown up, is one example.

The input data as well as the output can be stored for repeated use. The user can build a library of data and thereby simplify the pile group design commonly performed.

The PileCap.exe is designed to run under MS Windows XP or later.

Calculation method

The method used for the analysis is described in detail in Appendix A. In short, the calculation flow is as follows:

- for a single pile, calculate the forces and moments required at a pile top to give top the translations and rotations = 1 in the 6 possible directions (3 translations, 3 rotations). The coordinate system used is one with one axis parallel to the pile axis.

- transform these forces and moments to a coordinate system the axis parallel to the axis of the pile cap.
- the transformed forces and moments are moved to the origin of the pile cap

- the process is repeated for all piles. The contributions of all piles are summed in a stiffness matrix (6x6) at the pile cap origin. The element of the matrix expresses the force and moment respectively to give the pile cap a specified translation/rotation when loaded in a specific way.

- the loads (3 forces, 3 moments) are applied to the pile cap origin. Using the stiffness matrix assembled, the corresponding translations/displacements of the pile cap origin are calculated.

- the translation and rotation of a pile top is calculated, using the cap movements. These are expressed in the pile cap coordinate system. The cap is assumed perfectly rigid.

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- the pile top movements are transformed into the coordinate system of the pile. The pile direction and slope provides input data for the transformation.

- finally, the pile forces and moments are calculated using the relationship between movements and forces/moments established in the first item mentioned in the list.

- the process is repeated for all piles in the pile group. It is then assessed whether the movements and forces/moments are acceptable. Usually, the design must be modified and reanalysed a couple of times before a satisfactory result is obtained. Further, when the piles have been installed, there are usually deviations, as for example in slope and direction, who's influence must be analysed and maybe must also additional piles be installed, meaning further calculations.

It is obvious that the calculation process demands a vast amount of numerical calculations. Fortunately, this can be handed over to a computer, a PC doing Win XP. Doing so, the question arises how reliable are the results?

The computer uses a limited number if digits in its number crunching. In PileCap.exe all calculations are carried out in so called floating point double precision, meaning the results are as accurate as possible. Further, the reactions at each pile top are transformed and moved back to the pile cap origin, where they are compared to the applied loads (forces and moments). The difference is due to numerical truncations during all the calculations performed, and the residual vector (difference between applied and back calculated load) is a measure of the accuracy of the analysis.

It should be mentioned that numerical stability mirrors the physical stability of a pile group. For example, a pile group with only vertical piles when subjected to a horizontal force responds with an infinite translation, no matter how small the force is. Thus, such a pile group can not be analysed by the direct stiffness method described above. However, by cheating a little by applying a small amount of lateral soil resistance even such pile groups can be analysed.

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Start Page

At start, the PileCap.exe displays it's Start Up for, figure 1.

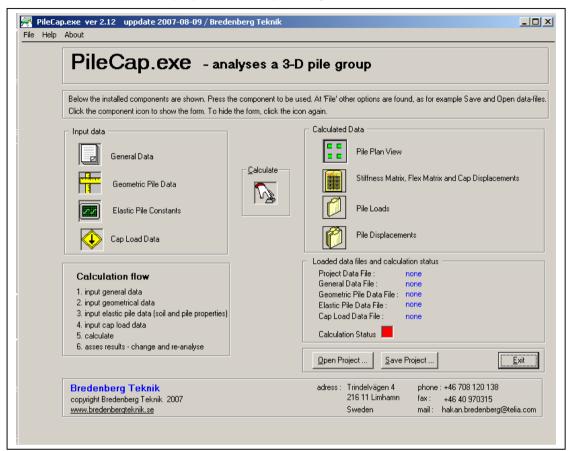


Fig 1. PileCap.exe start form

On this form there are several items aimed at guide the user through the design process. On the topmost bar the program version is displayed.

The menu row below shows the three items File, Help and About. The Help option simply points to this manual. The About choice can give detailed information about the system where PileCap.exe is run. This facility is useful in tracing eventual difficulties running the application. The content of the File option is described later.

Below the title row and some general information using the appl ication, there are two main frames containing Input Data and Calculated Data respectively. In between there is a command button making analysing the Input Data to happen. The function of each button is outlined later.

Next below there are two frames. The left one gives a schematic description of the usual calculation flow. The frame to the right contains the names of the input data files loaded. This frame also holds a status box, coloured green when a successful calculation has been performed using the Input Data loaded. When some Input has changed and a calculation has not yet been done, the status box is red.

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Below the right frame is a frame containing 3 command buttons. The rightmost button marked Exit offers the user the choice to stop the program. The leftmost button labelled Open Project opens up a dialogue, enabling the user to load a complete set of Input Data Files, see Figure 2.

Load Project Da	sta File	? ×
Leta i:	i: 🔁 Project Data 📃 🖛 🗈 📸 🛛	
Leta i Precent Skrivbord Mina dokument	i Project Data	
Mina nätverksplatser	Filnamn: Pålkomm R54 Uppu Filformat: Project Data Files (*.pcp) Avbr	

Fig 2. Dialogue box for loading a Project Data File (= a set of Input Data Files).

The function of this input box follows the standard Windows way to handle file input and is not further explained here.

The button Save Project opens up a similar dialogue box making it possible for the user to save the assembly of loaded Input Data files in to a Project Data File containing the assembly, figure 3.

				-	?
Spara i:	C Project Dat	a	- +	۱ 📩 🗈	•
	Pålkomm R54	ŧ.pcp			
	🖻 Test.pcp				
Recent					
Skrivbord					
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1					
Mina dokument					
Den här datorn					
	I				
S	Filnamn:	New Project			Spara
	r in an n	Inew Fiolect			
Mina nätverksplatser	Filformat:	Project Data Files (*.pcp)		•	Avbryt
natventspiatser					

Fig 3. Dialogue box for saving a Project Data File. (= an assembly of Input Data Files)

As is the case with the Input Dialogue, this is standard Windows stuff and no further explanation is given here. Consult the Windows manual, if necessary.

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www.bredenbergteknik.se		

The user can save his files where ever he wants in his system. However, it is recommended to maintain a systematic way of building a folder structure in order to make it easy to find and use stored data files. The picture below is shown as a recommend structure.

Load Project Dat	a File	<u>? ×</u>
Leta i:	🛅 PileCap 📃 🗢 🖻 🛗 🖽 -	
Senast använda dok Ø Skrivbord Mina dokument Den här datorn	Elastic Pile Constants General data Geometric Data ICONS LoadData Manual Project Data	
Mina nätverksplatser	Filnamn: Opp	ona
	Filformat: Project Data Files (*.pcp)	bryt

Fig 4 Recommended folder structure for storing data for Pile Cap calculations

If the user tries to open a Project Data File which is not such a file, an error message is displayed and the load is cancelled. The file extension (.pcp) is checked as well as a header written into every Project Data File saved.

When a valid Project Data File has been loaded, the name of the file is shown in upper case letters in the frame containing the names of the loaded data files. Further, the names of the individual input data files contained in the Project Data file are displayed in the frame.

Returning to the Start page, the bottom frame contains some information about Bredenberg Teknik, the copy right holder of PileCap.exe.

At the upper right corner on the Start form, the usual min, max, and stop buttons are found. If the stop button is pressed, a confirmation message box is shown, in order to avoid unintentional escape from the program.

The Start form can be minimised, maximised, moved and area modified the standard Windows way. These facilities are handy when arranging the different forms on the screen, when a pile group design is in progress.

Input Data

Input data are supplied by means of four different Input Data files for a Project. These Input files are :

File	Extension	Example
General Data	.gdt	Pgroup2.gdt
Geometrical Data	.geo	Group 9.geo
Material Data	.ela	New Piles.ela

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Cap Load Data	.ldt	LoadCase IV.ldt
Project Data	.pcp	ProjA11.pcp

The organisation of input data contained in the first four input files is described below together with the functionality of the respective input forms. The content of a project data file is an assembly of the data in the four basic files. The Project File Open/Save facilities makes it easy to load or save all project data in a single sequence, whereas the individual data in each of the four Input Data sets can be edited using the respective input form.

The user can save his files where ever he wants in his system. However, it is recommended to maintain a systematic way of building a folder structure in order to make it easy to find and use stored data files.

The file format for all Input Data files is simple ASCII -files (.txt-files). They can be edited by using any text editor, as for example Notepad. However, such editing may result in corrupted files, resulting in an error message when trying to make a calculation, or otherwise handling such a faulty file. Avoiding such editing eliminates the risk of such errors.

If the user tries to open an Input Data File which is not a valid such a file, an error message is displayed and the load is cancelled. The file extension is checked as well as a header written into every Input Data File saved.

Each Input Data form contains two buttons labelled Open and Save. They operate the same way as described above for Project Data files. The forms for the Input Data files can be minimised, maximised, moved and re-shaped the standard Windows way. This makes it possible to tailor the screen to the outfit that is best suited for the analysis being made. As for example, when inputting geometrical data describing the location and direction of individual piles, it is practical to make the geometrical input form and the pile plan view form visible side by side. Doing so, the user gets an immediate check that each pile are given correct coordinate and direction values.

If the user presses the X-button at the upper right corner of an Input Data form, a confirmation request message is shown, in order to avoid unintentional loss of data contained in the form, see fig 4 (as a bonus, you learn some Swedish, "Ja"=Yes and "Nej" = No,).

PileCap : (General Data 🔀
?	If this form is closed all data loaded in the form will be lost. Do you really want to close the form ?
	Ja Nej

Fig 4. Confirmation request when attempting to close an Input Data form

Should the form be closed this way anyhow, the data on the form are lost - however, if they have been saved, they can easily be retrieved using the Open button on all Input Data forms. It is recommended that input data are saved frequently.

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On each Input form there is also a status box. The status box turns green when the data in the text boxes have been Saved. If a box is changed the status box will turn red, alerting the user the data should be saved in order not to be lost in case the form is closed.

Below follows a description of the functionality of each of the four Input Data forms.

General Data Input form

Pressing the button General Data opens up an Input Form for such data, see Fig 5.

PileCap : Ge	neral Data	
Title :	Title of Project	Date : 07:21:08
Client :	Project Client	Time : 2007-08-21
Pile Cap ID :	Pile Cap ID	Set <u>T</u> ime and Date
Name :	Operator Name]
Comment :	Project Comments	
	Status Data Save : Dpen Save	

Fig 5. Form for Input of General Data.

The title bar contains the name of the input form, in this case General Data. Then there are a set of input text boxes where the user can input project data. The time and date, taken from the user system, can be put into the corresponding text boxes pressing the button labelled Set Time and Date.

The Open and Save buttons gives the user the option to Open and Save files containing General Data, by opening up dialogues as described above at the description of the functions of the Start Form. The function of the status box has also been described, as well as the control buttons in the upper rightmost corner.

Note that there is no exit button on the form. Usually the text boxes are filled and the data saved, and the form is then minimised and maybe kept in this shape until the session is ended. However, in case the user wants to close the form instead of minimising it, this ca be accomplished by clicking the X up right and confirm the intention to close the form.

Geometric Pile Data

The next input form to be filled is the form where the geometrical data for each pile in the group is typed. The form is activated by pressing the corresponding button in the input frame. The geometrical data input form is shown in fig 6.

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Pile nr	Pile ID	Type	x [mm]	y (mm)	z [mm]	n (n:1)	alpha (deg)	L (mm)	m (top)	Notations
	nr 1	1	1310				2011 Contract of the local sector of the local		0	
	vertikal 1	1	1450	-	0		0	10000	0	
	3 nr 2	1	1310	640	0	4.55	360	10000	0	<u>cap-origo</u>
	nr 3	1	470	-900	0	4.17	270	10000	0	xy
1	nr 4	1	650	790	0	5.00	90	10000	0	
	nr5	1	-520	-1030	0	4.55	270	10000	0	
	nr 6	1	-400	800	0	5.00	90	10000	0	plan view Ycap
1	3 nr 7	1	-1270	-1200	0	3.85	180	10000	0	the standards
!	vertikal2	1	-1420	90	0	0	0	10000	0	· · · · · · · · · · · · · · · · · · ·
11	nr 8	1	-1130	670	0	3.33	180	10000	0	cap-origo 2
										Zcap m=0 hinged pile top m=1 fixed pile top n = 0 for vertical pile

Fig 6. Form for Input of pile geometrical data

Data are typed into the cells in the grid. A completed cell input should be confirmed by pressing keyboard Enter. Piles can be inserted and deleted using the **Insert** and **Delete** buttons. The rightmost part of the form gives a description of the input data. This can be a handy reminder in case the user does not make pile group calculations very often, but this part of the form is usually removed during a design session, by resizing the form. The data to be filled in are as follows from left to right;

Pile ID : Any description the user wants to input to label the pile .

Type : a pointer to the material characteristics of the pile (see next form, material properties).

x,y,z : coordinates of the pile top, related to the cap coordinate system, note millimetres.
n : slope of the pile expressed as n:1, as for example 4:1. Input 0 (zero) for a vertical pile.
alpha: direction of the pile, degrees, projected to onto the horizontal plane.

L : pile length, where elastic compression is supposed to occu, note millimetres.

 \mathbf{m} : factor defining how the pile top is connected to the cap. Fixed is m = 1. Hinged is m=0. The function of the **Open** and the **Save** buttons have been described above, as well as the control buttons in the upper rightmost corner and the **Status box** down left.

In the control frame at the bottom of the form, the number of active piles in the grid is shown, to the right. To the left of this info, there is a button labelled **Draw plan**.

Clicking this button shows a plan view of the piles fed into the grid on the Geometrical Data input form, see figure 7.

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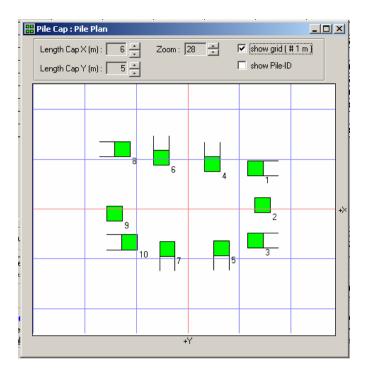


Fig 7. Plan view of piles in input grid

The plan view can be zoomed in or out in order to get a suitable picture resolution. A grid with 1 m grid line spacing can be turned on and of, as can the Pile ID:s also be. The center of the plan view is always the center of the cap. This is the horizontal projection of the pile cap origin, that is, where the loads acting on the cap are assumed to be situated (more about that below, at the description of the Cap Load form).

The cap dimensions can be changed using the leftmost up/down buttons above the view. However, these dimensions have no influence of the calculation results.

Pile and soil material data

When the geometrical data of each pile has been defined, it's time to define the material properties of the piles and also the soil surrounding the piles. This is done by calling up the form called Pile Elastic Constants, see figure 8.

In order to calculate the cap response of external loads, the sum of all piles contributions to the cap stiffness must be assembled. Therefore the relationship between forces and moments and the corresponding translations and rotations for each pile top must be evaluated. This is what the form in fig 8 is for.

The values used in the calculation are the ones in the uppermost boxes. Since there are 6 degrees of freedom (3 translations and 3 rotations) for the pile top, there should be 6 boxes, but since symmetrical conditions for each pile top are assumed the number of stiffness factors are reduced. The stiffness factor kij defines the force (moment) required in direction "i" to give the pile to a translation (rotation) equal to 1 length unit (one radian) in direction "j". The direction of "i" and "j" are defined in the appendix describing the numerical method for the analysis. For example, k33 is the force applied to the pile top which causes a unit compression of the pile at the pile top.

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The form shown in fig 8 is created to simplify the evaluation of the factors kij. Often the soil resistance around the piles is neglected and then only factor kij to be considered is the axial stiffens. In the form, a different cross section shape, the side/diameter length and the pile material can be selected and the corresponding stiffness calculated. The E -modulus can be selected as standard values, or modified using text box input.

In case the user wants to consider soil resistance around the piles (recommended), PileCap.exe offers the option to do that by the input of a soil E-modulus (see rightmost input frame). The kij values are then calculated assuming the piles are equivalent to semi infinite beams in a Winkler (linear spring) environment.

In case another way to calculate the stiffne ss factors are preferred by the user, maybe factors evaluated from a load test, this can be accomplished by typing in the factors and then upload them by pressing the button **Upload all boxes**. Upload is done to the row pointed at.

It should be mentioned that it is possible to use different stiffness properties for every pile in a group. However, usually it is assumed that all piles have the same stiffness characterises.

Pile Cap : Elastic Pile Constants			X
Type r Type ID k11=k22, kN/m k15=k51, kNn 1 BtgPåle 0 0	n/rad k24=k42, kNm/rad k3 0 -0	33, kN/m/m k44=k55, kNm/ra 2187000	d k66,kNm/rad/m 0 0
Status Data Save :	Insert Type	<u>D</u> elete Type	Number of types : 1
Single pile stiffnes factors kij k11=k22 : k15=k51 [kN/m] [kNm/rad] 321 0 Force/moment kij gives displacement/rotation =1 in direction 'T' when applied in direction 'T'.	k24=k42 : [KNm/rad] [note minus-sign] 0 To upload single boxes to grid	k33 : k44=k [KN/m/m] [for 1 m pile] [kN/ra 1159725] d use CtrI+C use CtrI+V. <u>U</u> ploa	[kNm/rad/m]
Pile cross section Image: square, massive Image: square, massive Image: cross section Image: cross section <td>Calculated section data Inner- diameter, mm : 235 Area.cm2 : 552 Moment of - inettia , cm4 : 25415 Twist moment- factor, cm4 : 3558</td> <td>concrete E steel steel and concrete</td> <td>-moduli moduls pile, GPa : 21 * modulus soil, kPa : 200 * sloulated Elastic Length, m : 4.15</td>	Calculated section data Inner- diameter, mm : 235 Area.cm2 : 552 Moment of - inettia , cm4 : 25415 Twist moment- factor, cm4 : 3558	concrete E steel steel and concrete	-moduli moduls pile, GPa : 21 * modulus soil, kPa : 200 * sloulated Elastic Length, m : 4.15

Fig 8 Pile and soil material data, elastic properties

The Open and Save as also the control buttons up to the right have the same functionality as is the case for the other Input Forms.

Pile Cap Load Data

The loads acting at the cap are input into the last of the four input forms, se figure 9.

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🚸 PileCap : Lo	oad Data								
Load Case	Case ID LastFall A LastFall B	Fx [kN] -75 0		1940	Mx [kNm] -284 0	902	Mz (kNm) 0 0		Notations for loads : Cap origo Fx
Status Data	Save:	<u>Open S</u>	ave	Insert Row	Delete	Row L	oad Cases :	2	Fz Positive axes for forces Fx,y,z are shown. Positive direction for moments Mx,y,z : right

Fig 9. Pile Cap Load form.

The loads are related to an origin usually chosen as the gravity centre of the cap. There are 6 generalised forces, 3 forces and 3 moments.

The directions of the positive axis are shown in a picture to the right of the input data grid. Vertical loads (z-axis) are parallel to the direction of gravity, usually. If so, the x - y - ydirections establishes the horizontal plane. The respective moment is assumed positive turning right wise when looking in the respective axis.

In the form, info about sign conventions is given in the picture to the right of the input grid. This part of the form can be removed when the user has understood the force direction rules. In case he will return to this form without a clear memory of the direction signs used the picture may serve as a reminder.

Any number of load combinations can be loaded. The active number is shown at the right in the control frame below the grid.

The functionality of the **Open** and the **Save** buttons, as well as the **Status** box different states (green / red) has bees described above.

Menu bar, the File option

As mentioned above, the File option at the menu bar have a special function. Pressing the File item, a drop down list is presented, as shown I fig 10.

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Help Abou	JL JL		
lew Pile Group	2.00		
Open Projecct Open General Open Geometr		EXE - analys	es a 3-D pile group
Open Elastic Pi	ile Data File		ponent to be used. At 'File' other options are found, as for example Save and Open data-file:
		p show the form. To hide the fo	rm, click the icon again.
ave Project Data File			Calculated Data
ave Project Data File ave General Data File ave Geometric Pile Data File ave Elastic Pile Data File ave Cap Load Data File		r0	alculate File Plan View
201200	Data File	e Data	Stilfness Matrix, Flex Matrix and Cap Displacements
Print		-	
Xit		onstants	Pile Loads
	Cap Load I	Data	Pile Displacements
(Calculation flov	•	Loaded data files and calculation status Project Data File : none
1	I. input general data		General Data File : none Genmetric Pile Data File : none
	2. input geometrical d	ata	Elastic Pile Data File : none
		ta (soil and pile properties)	Cap Load Data File : PÅLKOMM R54 TVÅ LASTFALL LDA
	 input cap load data calculate 	1	Calculation Status
	o, calculate 6. asses results - char	nce and re-analyse	
			Open Project Save Project
	redenberg Tek		adress : Trindelvägen 4 phone : +46 708 120 138 216 11 Limbarran fav:
	pyright Bredenberg T ww.bredenbergteknik		21611 Limhamn fax∶ +4640.970315 Sweden mail∶ hakan.bredenberg@telia.com

Fig 10.The File Option at the menu bar

From this list it is possible to make the same type of Open/Save operations as described above.

Any attempt to load an input data file not compatible with the type assumed, will result in an error alert from the program. It is recommended that input data files are created using PileCap.exe, and that the input data file are kept in the format used by PileCap.exe.

Calculated data

O k, now the PileCap.exe has got enough info about your pile group. So let us analyse it ; easiest part of the manual . Jus the press Calculate button between the two frames at the top of the start6 page.

When you do so, a considerable amount of numerical analysis is done for you. It involves matrix inversion, pivoting, and lots of number crunching. In case anything goes wrong during the calculation, an error message is generated. However, if the input data files have passed the tests the program generates and the pile group is reasonably stable in a physical sense, the calculation should not raise any error flags. A successful calculation gives a green status light for the status box at the calculation frame at in the Start form.

Stiffness matrix, flexibility matrix, cap displacements

The primary calculation result is the stiffness **S** matrix for the pile cap. It is a 6*6 symmetrical, positive definite, matrix containing the forces and moments required to translate and rotate the cap one length unit for translation and one radian for moments, respectively. The 6*6 order of the matrix reflects the 6 degrees of freedom (3 translations, 3 rotations) the cap can do. The element **S**ij of the stiffness matrix is thus the force (moment)

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required to give a translation (rotation) equal to 1 length (rotation) unit. The stiffness matrix is symmetrical.

i = 6 -255787 559625538 -11047.6866849776 2452.48016366653 -107557.16219530 -4652.67030410219 -12552.33972596 2428032.21424039 -46845.991919991 -46845.991919991 -46845.9919166 -7.906279663610326.07 2.324096525302376.07 2.324096525302376.07 2.324096525302376.07 2.324096525302376.07 2.324096525302376.07 2.324096525302376.07 2.324096525302376.07 2.324096525302376.07 2.324096525203276.07 2.324096525203276.07 2.324096525203276.07 2.324096525203277.07 2.3240965252032.07 2.32409652721.5721.69 -1777773172448496.06 3.759377.42627936.07 2.840517381989986.07 2.840517381989986.07 2.840517381 2.840517381
-11047.8866849776 2.452.40016968553 -107557.162165308 -4652.67030410219 -12552.33972506 55367.5512644762 2428032.21424033 -46845.95191919891 -46845.9519139301 45015.6512544762 2428032.21424033 -46845.95191919891 -46845.9519139301 43015.8615965838 -46845.951913691 43015.86159651032 -46845.923726 -7.906279663610325.07 2.32406525202375.07 5.906433065951365.06 9.28206042935226.09 -9.960000897931315.06 6.43827227137216.09 -1.779779172484826.06 8.759977.42527836.07 2.840517381089896.07
-107567162196308 -4652.57030110219 -125523.339972586 59367.6512644782 2428032.21424039 -46845.391919991 -46845.991919991 49015.861595533 -46845.991919991 49015.861595533 -45845.991919991 49015.861595533 -23240955530237E-07 5.90643306955196E-06 -323268042935224.08 -9.0600098793131E-08 -6.43827227135721E-09 -1.7797731724849450-06 -6.75995774252738E-07 2.4045173181989954-07
125523.338972586 59367.6512644782 2428032.21424033 46845.391919891 46845.391919891 49015.861595638 439126776411551E.06 -7.90627966361032E.07 2.3240652520237E.07 5.90643306356196E.06 9.2826604293523E.09 -9.906000089733131E.08 6.43827227135721E.09 -1.7797791724843E.06 6.43827227135721E.09 -1.7797791724843E.06 8.7599774252783E.07 2.84051738168093E.07
2428032.21424039 -46845.991919891 -46845.991913691 45015.6615965638
46845.991913691 49015.8615965838
4.31126776411551E-06 -7.90627966361032E-07 2.3240955550237E-07 5.90643005956196E-06 9.282604293525E-08 -9.09500088793131E-08 6.4382722715721E-09 -1.77977917244949E-06 8.76997174262783E-07 2.84051738198099E-07
4.31126776411551E-06 -7.90627966361032E-07 2.3240955550237E-07 5.90643005956196E-06 9.282604293525E-08 -9.09500088793131E-08 6.4382722715721E-09 -1.77977917244949E-06 8.76997174262783E-07 2.84051738198099E-07
2.3240865250237E-07 5.90548306956196E-06 9.2265804235528-08 -9.0660008873313E-08 6.43827227135721E-09 -1.77977917244849E-06 8.76997174262783E-07 2.84051738189898E-07
2.32408652520237E-07 5.90548306956196E-06 9.22605804233523E-08 9.06600088733131E-08 6.43827227135721E-09 1.77979717244849E-06 8.76997174262783E-07 2.84051738108898E-07
6.43827227135721E-09 -1.77977917244849E-06 8.76997174262783E-07 2.84051738188889E-07
8.76997174262783E-07 2.84051738188889E-07
2.84051738188888E-07 2.2624192492775E-05
-1 59904997365035E-17 3 27293530819062E-18
-1.59904997365035E-17 3.27293530819062E-18 5.19570009845788E-18 -5.07406616723216E-17
5.19570009845788E-18 -5.07406616723216E-17
5.19570009845788E-18 -5.07406416723216E-17 1 -8.56519716263549E-18 6.31039545704454E-18 1 off' (rad*1000) rol2 (rad*1000)
5.1957009845788E-18 -5.07406616723216E-17 1 -8.58519716263549E-18 6.31039545704454E-18 1
2.03610838652173E-18 1.458251

Fig 11 Stiffness matrix, flexibility matrix and pile cap displacements

The stiffness matrix is made visible by clicking the Stiffness button on the Start form. An example is shown in fig 11.

Below the stiffness matrix on this form, there are also the flexibility matrix, the check matrix and the cap displacements.

The flexibility matrix is the inverse matrix \mathbf{S}^{-1} of the stiffness matrix. The check matrix is calculated by the matrix multiplication

s⁻¹ **s**

which shall yield the 6*6 identity matrix **I**, in which all diagonal elements are equal to 1 and all others 0. A look at figure 11 shows that the off diagonal elements of the check matrix are not really zero, but instead something like 10⁻⁶ to 10⁻¹⁹. The reason is truncation errors, since real number multiplication is carried out using a limited number of digits. However, a number that small is in most pile cap calculations something that can be considered zero.

As can be seen from fig 11, the results are given with lots of decimals. In order to make grids presenting the output easier to read, the number of decimals can be decreased using the

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up-down button at the upper right corner of the form. For example, letting the number of decimals tick down to zero gives the example shown in fig 11 the following look :

Lionio		" Corresponding to cap displacement					-
	i=1	i=2	i=3	i = 4	i=5	i=6	
j=1							
i=2		0 38793					
i=3							
j = 4							
j=5				-125523			
i = 6	61	79 2452	-4653	59368	-46846	i 49016	
lexib	ility Matrix [S ⁻¹] [Inverse o	l StiffnessMatrix)					
	i=1	i=2	i=3	i=4	i=5	i=6	
= 1		0 0	0	0	0	0	
=2		0 0	0	0	0	0	
= 3		0 0	0	0	0	0	
= 4		0 0	0	0	0	0	
= 5		0 0	0	0	0	0	
= 6		0 0	0	0	0	0	
	i=1	i=2		i=4	i=50	i=60	
= 1			0	0	0	0	
= 2		0		0	0	0	
		0 0		1	0	0	
				0	1	0	
				0	0	1	
= 5					0		
= 4 = 5 = 6		JU	0				
= 5 = 6			0				
= 5 = 6	placements { u } = [S ⁻¹]{F}		Z (mm)	roK (rad*1000)	rofY (rad*1000)	rot2 (rad*1000)	T
= 5 = 6 p disp	placements { u } = [S ⁻¹]{F} C	ap load vector 6x1 = {F}		roi× (rad*1000)	rołY (rad*1000)	rof2 (rad*1000)	0

Fig 12. Same as fig 11, but with zero decimals

In the 3:d grid, the check matrix, it is now seen that the diagonal elements are all 1 and the others zero. However, information about the elements of the flexibility matrix (not often required, though), and the pile cap displacements require more decimals than zero in order to find out actual values. This is easily done with the up-down button up right.

The last grid shows the translations (millimetres) and the rotations (radians) of the cap. The values are calculated as

$$\{u\} = S^{-1} \{F\}$$

where **{F}** is a column vector holding the forces (F1, F2, F3) and the moments (M1, M2, M3) acting at the cap origin. As for the grids above, the numbers of decimals shown are set by means of the up-down button at the upper right corner of the form. The first column of the grid gives the number of the load case corresponding to the loads and displacements.

Pile loads, pile displacements

Next two calculation-show buttons that can be clicked at the Start Up form makes the pile load grid and the pile displacement grid visible, se fig 13 and fid 14. Again, the number of decimals can be set by an up-down button situated upper right. Note that the moments and the rotations for hinged pile-cap connections are 0, as in the figures shown.

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In the pile force grid, the maximum and the minimum axial force are shown in bold red and bold blue font, respectively.

ile Loads ·	forces	(kN) and bending mom	ents (kNm)		red text = maximu blue text = minimu		of decimals : 7 🛓
oad Case	pile #	Fx, kN	Fy, kN	Fz, kN	Mx, kNm	My, kNm	Mz, kNm 🔺
1	1	0.0000000	0.0000000	46.9592532	0.0000000	0.0000000	0.00(
1	2	0.0000000	0.0000000	13.8340653	0.0000000	0.0000000	0.00(
1	3	0.0000000	0.0000000	193.3484524	0.0000000	0.0000000	0.00(
1	4	0.0000000	0.0000000	263.2968858	0.0000000	0.0000000	0.00(
1	5	0.0000000	0.0000000	39.1761214	0.0000000	0.0000000	0.000
1	6	0.0000000	0.0000000	368.3302584	0.0000000	0.0000000	0.000
1	7	0.0000000	0.0000000	177.3517415	0.0000000	0.0000000	0.00(
1	8	0.0000000	0.0000000	148.3697006	0.0000000	0.0000000	0.00(
1	9	0.0000000	0.0000000	422.4350475	0.0000000	0.0000000	0.000
1	10	0.0000000	0.0000000	310.4318916	0.0000000	0.000000	0.000
2	1	0.0000000	0.0000000	166.8386689	0.0000000	0.0000000	0.000
2	2	0.0000000	0.0000000	155.3356178	0.0000000	0.0000000	0.000
2	3	0.0000000	0.0000000	264.2054495	0.0000000	0.0000000	0.00(
2	4	0.0000000	0.0000000	185.1293448	0.0000000	0.0000000	0.00(
2	5	0.0000000	0.0000000	197.2229900	0.0000000	0.0000000	0.000
2	6	0.0000000	0.0000000	205.2500033	0.0000000	0.0000000	0.00(
2	7	0.0000000	0.0000000	247.5638322	0.0000000	0.0000000	0.00(
2	8	0.0000000	0.0000000	134.1058385	0.0000000	0.0000000	0.000
2	9	0.0000000		281.3829326	0.0000000	0.0000000	

Fig 13. Pile reaction forces and moments (at the pile-cap interface)

e Dis	splacer	nents - translations (mm	n) och rotations (rad*10	00)			number of decimals : 2	
tfall	påle #	X, mm	Y, mm, kN	Z, mm	X-rot, rad*1000	Y-rot, rad*1000	Z-rot, rad*1000	-
1	1	1.93298228477667	-3.96685481160048	0.214719950474373		0	0 0	
1	2	1.88607532118483	-3.95766047169199	6.32558997141786E-02		0	0 0	
1	3	1.6883739722003	-3.96685479392507	0.884080715127865		0	0 0	
1	4	3.84734173027533	1.93992522090811	1.20391908759087		0	0 0	
1	5	-4.12544151035257	-1.82894111082858	0.179131785109075		0	0 0	
1	6	3.81442829025342	1.94846247985035	1.68418042236706		0	0 0	
1	7	-4.32212282045813	-1.82828439470616	0.810936175061846		0	0 0	
1	8	-2.20086317239311	4.13628656850139	0.6784165549617		0	0 0	
- 1	9	1.87491120150133	-4.14613708046639	1.93157314812329		0	0 0	
1	10	-2.34411524458805	4.12709257618798	1.41944166240533		0	0 0	
2	1	0.974934310247075	-1.00496344204229	0.762865427118905		0	0 0	
2	2	1.24876432081589	-1.03043203738305	0.710268028345663		0	0 0	
2	3	1.14716633012141	-1.00496349100599	1.20807247135949		0	0 0	
2	4	0.67331341661442	1.09958978762142	0.846499061931316		0	0 0	
2	5	-1.08277987185028	-1.4070346952022	0.90179693664226		0	0 0	
2	6	0.481825679067955	1.07594021038559	0.938500243636211		0	0 0	
2	7	-0.934017426041508	-1.4088539042439	1.13197911365554		0	0 0	
2	8	-1.23896101715748	0.535609686067542	0.613195420803994		0	0 0	
2	9	1.27969072383485	-0.508321586415794	1.28661606129576		0	0 0	

Fig 14. Pile translations and rotations (at the pile-cap interface)

Like the input data forms, a warning is given if the user presses the close button for any of the forms presenting calculated data. Should however an unintentional close be done, this is not critical, since the calculation can be repeated just by pressing the Calculation b utton on the Start form again.

Equilibrium check

On screen there are a lot of pile loads and pile top displacements displayed. How do you know they are accurate? Well, actually you ca'nt unless you get some support. If there are huge and obvious errors it's safe to conclude the calculated data are not reliable. But if there are errors say 10 - 30 %, it's generally not possible to figure out something is wrong.

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The stiffness matrix, the flexibility matrix and the calculation of the product of these will give one indication of the reliability of the results displayed. The fact that the matrix inversion was possible to carry out means that the pile group analyzed is stable from an engineering point of view – that is, there is no direction in the cap coordinate system where a infinitely small force (moment) will give a huge (unlimited) translation (rotation).

Further, as a check maybe easier to understand, an equilibrium check is made, se figure 15.. All the calculated forces and moments at each pile top are rotated into the cap coordinate system and the corresponding force and moment at the cap origin is calculated. The contribution from each pile is accumulated, and when all piles have been calculated this way, the sum is compared to the loads applied to the cap. The difference is calculated and presented. An example is shown in fig 15.

_astFall A	-75.00	-98.00	1940.00	-284.00	902.00	
LastFall B	0.0	0.00	2000.00	0.00	0.00	0.00
			ile venetiene merued te	cap origin)		
	lated Cap Loads and					
LoadCase	F'x, kN	F'y, kN	F'z, kN	M'x, kNm	M'y, kNm	M'z, kNm
LoadCase .astFall A	F'x, kN -7	F'y, kN	F'z, kN 1940	M'x, kNm -284		1.4210854715202E-14
Back Calcul LoadCase LastFal A LastFal B	F'x, kN -7	F'y, kN 5 -98	F'z, kN 1940	M'x, kNm -284	902	1.4210854715202E-14
LoadCase LastFal A LastFal B Difference =	Fx, kN 77 7.105427357601E-19 7.105427357601E-19	Fy, kN 	F2, KN 1940 2000	M'x, kNm -284 8.5265128291212E-14	902 -1.13686837721616E-13	1.42108547152022-14 0
LoadCase LastFal A LastFal B Difference = LoadCase	Px. kN -77 7.105427357601E-15 abs[Given] - abs[Baa	Fy, IN -98 3 33452188793936E-14 3 33452188793936E-14	Fz. KN 1940 2000 Fz - Fz JAN	M'x, kNm -284 8.5265128291212E-14 Mx - M'x , kNm	902 -1.13686837721616E-13 My - M'y , kNm	1.42108547152022-14 0 Mz - M'z , kNm
LoadCase LastFal A LastFal B	Fx, kN -77 7.105427357601E-15 abs[Given] - abs[Bar Fx - Fx, JN -2.8421705430404E-14	Fy, IN -98 3 33452188793936E-14 3 33452188793936E-14	F2, KN 1940 2000 F2 - F2, JAN 0	M'x, kNm -284 8.5265128291212E-14 Mx - M'x , kNm 0	902 -1.13686837721616E-13 My - M'y , INm 2.27373675443232E-13	1.4210854715202E-14 0 Mz - M*z , INim -1.4210854715202E-14

Fig 15. Equilibrium check. Cap Loads, back calculated loads and differences.

Form fig 15 it there are errors in the magnitude of 10^-14 or less. This is pretty close to zero, which is the correct value of course. For engineering purposes such a small resulting truncation error means that the results of the calculation are validated. The reason exact zero is not obtained is that the computer uses a limited number of digits doing the math.

The double precision math, and the numerical method used a re very powerful when dealing with marginally stable pile groups. If you are interested in testing the abilities of PileCap.exe in this respect, you will find that un unstable configuration of piles (as for example only a few vertical piles with hinged connection to the cap) cap be analyzed applying only a very small soil support to one of the piles in the group.

A division by zero situation (e g a horizontal pile) can not yield a result, by there will be no crash to your desktop, and the error messages are not intimidating. Not by purpose, anyhow.

The results of the equilibrium check are displayed by pressing the Equilibrium Check button on the Startup form.

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Pile Cross Section capacity check

When the axial load and the bending moments at the pile top has been calculated, it is time to check whether the pile cross section can take these load effects. Usually, this is checked using an interaction formula,

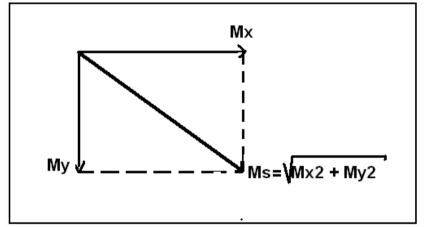
Ns/Nd + Ms/Md <=1

where Ns = axial load Nd = axial load capacity Ms = bending moment acting together with the axial force Md = moment capacity

Usually reduction factors are added in order to consider local buckling and other effects affecting the bearing capacity of the cross section. However, the basic formula for a section subjected to both a normal force (for piles usually a compressive force) and a connected bending moment is the one shown above.

For a 3D pile group, the individual piles will usually have bending moments in three directions, two bending moments Mx and My acting around the two horizontal axis's x and y respectively and a twisting moment Mz. The moment Ms to feed into the interaction formula is the resulting moment from Mx and My, that is

 $Ms = sqr(Mx^2+My^2)$



That is, the resultant of the two moments if they are looked at as vectors, se fig 16.

Fig 16. Resulting bending moment from Mx and My

The influence of the twisting moment Mz must also be considered when the cross section is checked. The moment Mz gives shear stresses in the section. These stresses are combined with stresses caused by compression/tension and the other two bending moments. National building codes have rules how to take such situations into consideration.

PileCap.exe has a facility to make the calculation of the necessary pile cross section capacity. This option is activated by pressing the Cross Section button on the Start Up form. A new form is the 'displayed, se fig 17.

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	Fx	Pile cross section	n capacity : Nd	= axial capacity,	kN Md =	moment capacity	, kNm		
1		$Hmax = \sqrt{Fx^2 + F}$	y ² Mmax =	4x ² + My ²	Nd, kN = 410	0.00 Md, k	Nm = 50.00		Update
LoadCase	Pile nr	Hmax ,kN	N, kN	Mmax, kNm	N/Nd	M/Md	N/Nd+M/Md	Pile ID	LoadCaseID
1	1	0.00		0.00	0.11	0.00	0.11	nr 1	LastFall A
1	2	0.00	13.83	0.00	0.03	0.00	0.03	vertikal 1	LastFall A
1	3	0.00	193.35	0.00	0.47	0.00	0.47	nr 2	LastFall A
1	4	0.00	263.30	0.00	0.64	0.00	0.64	nr 3	LastFall A
1	5	0.00	39.18	0.00	0.10	0.00	0.10	nr 4	LastFall A
1	6	0.00	368.33	0.00	0.90	0.00	0.90	nr 5	LastFall A
1	7	0.00	177.35	0.00	0.43	0.00	0.43	nr 6	LastFall A
1	8	0.00	148.37	0.00	0.36	0.00	0.36	nr 7	LastFall A
1	9	0.00	422.44	0.00	1.03	0.00	1.03	vertikal2	LastFall A
1	10	0.00	310.43	0.00	0.76	0.00	0.76	nr 8	LastFall A
2	1	0.00	166.84	0.00	0.41	0.00	0.41	nr 1	LastFall B
2	2	0.00	155.34	0.00	0.38	0.00	0.38	vertikal 1	LastFall B
2	3	0.00	264.21	0.00	0.64	0.00	0.64	nr 2	LastFall B
2	4	0.00	185.13	0.00	0.45	0.00	0.45	nr 3	LastFall B
2	5			0.00	0.48	0.00	0.48		LastFall B
2	6		205.25	0.00	0.50	0.00	0.50	nr 5	LastFall B
2	7	0.00	247.56	0.00	0.60	0.00	0.60	nr 6	LastFall B
2	8	0.00	134.11	0.00	0.33	0.00	0.33	nr 7	LastFall B
2	9	0.00	281.38	0.00	0.69	0.00	0.69	vertikal2	LastFall B
2	10	0.00	204.49	0.00	0.50	0.00	0.50	nr 8	LastFall B
2	10	0.00	204.49	0.00	0.50	0.00	0.50	nrö	Lastrali b

Fig 17. Pile cross section check form. The capacity of pile 9 seems too low (red).

On this form, the values of resulting bending moment Ms, as outlined above, is calculated. Then the interaction quotient is calculated. If >= 1, it is displayed in red. The value is calculated for every pile in the pile group and also for each load case. The capacity values Nd and Md can be input to boxes above the grid, se e fig 17. The calculation is invoked by pressing the UpDate button. A status box displays red when a change input data of Nd or Md ahs been made and an Update calculation is to be done.

In case a value >1 is displayed, the capacity of the pile section must usually be increased. In case of a steel tube pile, this can be accomplished by increasing the diameter and/or the wall thickness of the tube, for example.

Further cross section examination

The horizontal forces Fx and Fy at the pile head will cause a bending moment in the pile at sections below the pile/cap interface. The resultant of these two forces (Hmax) is calculated in the same manner as for the bending moments, that is

Hmax = $sqr(Fx^2 + Fy^2)$

See also fig 16. The resultant Hmax is calculated in PileCap.exe and displayed in column 3 on the form shown in figure 17.

The bending moment at the pile top will decrease downwards the pile, whereas the bending moment from the horizontal force Hmax will increase downwards until a peak I reached, and after that the moment will decrease. The value of this moment depends on the soil and the pile properties. The two bending moments will interact and a maximum is reached at some

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depth. This is the section of peak interaction factor. The shear is zero where the bending is max.

Note also that the interface section between the pile and the cap must be analyzed taking the shear Fs on the stresses into account, together with the stresses caused by the resulting bending and the twisting moment and the axial force. In practice, this can be difficult to validate for some types of piles.

This dilemma is sometimes 'solved' by assuming that the piles are ball point connected to the cap, and thus no moments will occur. Further the influence of the soil is neglected. With these assumptions, the piles are loaded by an axial force only, making the analysis easier to carry out. Considering that both numerical models and calculation tools are c ommonly available, such an over simplification appears hardly justified. There is no longer any reason to treat piles different than other structural members for building structures , as far as analysis of structural issues are considered.

A closer look down along the pile

In order to simplify the calculation of the peak bending moment, the option "Pile Bending Moment" is given in the Cross Section Analyse form shown in fig 17. The bending moment down along the pile is made, se fig 18. The calculated bending moments are assuming the pile acts as a beam in an semi infinite elastic surrounding.

Fig 18 Bending moments caused by a moment and a horizontal force at the pile top

The resulting horizontal force and the resulting bending moment are assumed to act in the same vertical plane. The corresponding bending moments are shown in figure 19. These results ca be used for design of pile groups and other engineering structures possessing same characteristics as a pile group, a for example elements bolted together in order to avoid welding.

Modifying Input data

When an calculation run has been made, modification of input data, as for example additional piles, changing cap load data, changing pile and soil data, and so on can be analysed without saving any data. Bring up the form(s) where the changes are required, hit Enter to get rid of any yellow grid markers, and make a new calculation. Used this way, PileCap.exe is a very powerful design tool.

Presentation of results. Distribution of results from PileCap.exe.

In order to print out data on paper, or sending them to some application converting data into Adobe pdf-format för mailing them somewhere, the use of some application taking care of information on screen is assumed.

One solution not demanding any software other than components supplied with a standard Windows installation is as follows:

- 1. Bring up the forms to be printed
- 2. Press the PrintScreen command.
- 3. Start Paint and press Paste to copy the screen image to the Paint canvas.
- 4. Cut the part you are interested in of the pasted image
- 5. Start new, don't save, and paste. The copied part will appear alone.
- 6. Print, mail, and store the image, as required.

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In order to make a pdf of the image, you need a pdf-converter. Many freeware such applications ca be found for free on the internet, or be purchased from Adobe in case you will go for the full functionality of the pdf-file format.

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